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European Technical Assessment ETA-13/0026 of 24/04/2017

I General Part

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011: ETA-Danmark A/S

Trade name of the construction product:

BB Stanz- und Umformtechnik GmbH post bases

Product family to which the above construction product belongs:

Three-dimensional nailing plate (Post bases)

Manufacturer:

BB Stanz- und Umformtechnik Nordhäuser Str. 44 D-06536 Berga Tel. +49 34651 2988 0 Fax +49 34651 2988 20 Internet www.bb-berga.de

Manufacturing plant:

BB Stanz- und Umformtechnik

This European Technical Assessment contains:

59 pages including 2 annexes which form an integral part of the document

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of: Guideline for European Technical Approval (ETAG) No. 015 Three Dimensional Nailing Plates, April 2013, used as European Assessment Document (EAD).

This version replaces:

The ETA issued on 2013-02-01 and expiry on 2018-02-01 Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and should be identified as such.

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II SPECIFIC PART OF THE EUROPEAN TECHNICAL ASSESSMENT

1 Technical description of product and intended use

Technical description of the product

The BB Stanz- und Umformtechnik post bases are made from 2.5 mm to 8.0 mm thick steel plates in combination with steel tubes and rods. The post bases are produced of steel grade S235JR according to EN 10025-2:2005-04 with a minimum characteristic yield strength of $R_{eH} = 235 \text{ N/mm}^2$ and a minimum characteristic tensile strength of $R_m = 360 \text{ N/mm}^2$ and steel grade DD11 according to EN 10111-2:2008 with a minimum characteristic yield strength of $R_{eL} = 170$ N/mm². The steel tubes are produced of steel grade S195T according to DIN EN 10255:2007 with a minimum characteristic yield strength of R_{eH} = 195 N/mm² and a minimum characteristic tensile strength of $R_m = 320 \text{ N/mm}^2$. The steel rods are produced of steel grade S235JRC+C according to EN 10277-2:2008-06 with a minimum characteristic yield strength of $R_{p0,2} = 260 \text{ N/mm}^2$ and a minimum characteristic tensile strength of $R_m = 390 \text{ N/mm}^2$. The threaded rods correspond to property class 4.8 according to DIN EN ISO 898-1:2009-08.

For the connections with metal fasteners dowels $\emptyset10$ mm (S235) or bolts $\emptyset10$ mm (4.6) and screws $\emptyset12x120$ mm, $\emptyset10x100$ mm, $\emptyset10x80$ mm; $\emptyset10x50$ mm or $\emptyset6x80$ mm according to EN 14592 (DIN 571 and thread according to DIN 7998) with a minimum characteristic tensile strength of $R_m = 360$ N/mm² or according to an ETA are used. The screws shall be driven into predrilled holes according to EN 1995-1-1, 10.4.5 or respectively according to the ETA of the screws.

Dimensions are shown in Annex A and B.

2 Specification of the intended use in accordance with the applicable EAD

The intended use of the post bases is the support of timber columns and posts as load-bearing elements, where requirements for mechanical resistance and stability and safety in use in the sense of the Basic Works Requirements 1 and 4 of Regulation (EU) 305/2011 shall be fulfilled.

The static and kinematical behaviour of the timber members or the supports shall be as described in Annex B.

The timber posts may be of solid timber of strength class C24 or better according to EN 338:2016 or of glued laminated timber according to EN 14080:2013.

Minimum dimensions for the post have to be considered (Annex A).

The post base shall be installed as pictured in the drawings. The cross-section of the timber column shall be positioned centrically and with the end grain plane on the base plate. The end grain of the timber post must in general be plane on the base plate of the post base. Post bases types H have a distance between the end grain of the timber post and the base plate of the post base up to 10 mm due to constructive wood preservation.

The maximum distance between the foundation and the base plate of the post base is given in Annex A, table A 1

Annex B states the load-carrying capacities of the post bases for solid timber of strength class C24 according to EN 338:2016. The design of the connections shall be in accordance with Eurocode 3 and Eurocode 5 or a similar national code. The anchorage of the post base in the foundation and imperfections exceeding the assumptions in Eurocode 5, 5.4.4 are not part of this ETA.

The post bases are for use in timber structures subject to the service classes 1, 2 and 3 of Eurocode 5 and for connections subject to static or quasi-static loading. In service class 1 and 2 the corrosion protection is given by hot-dip zinc coating Z275 according to EN 10147 or zinc coating according to EN1461 with a minimum thickness of 8 μ m. In service class 3 the corrosion protection is given by hot-dip zinc coating Z350 according to EN 10147 or zinc coating according to EN1461 with a minimum thickness of 55 μ m.

The metal fasteners must also have a zinc coating according to EN ISO 2081 corresponding to the relevant service class 1, 2 or 3 of EN 1995-1-1. Galvanic zinc coating of the post bases is only suitable for service classes 1 and 2.

The scope of the hangers regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions.

The provisions made in this European Technical Assessment are based on an assumed intended working life of the post bases of 50 years.

The indications given on the working life cannot be interpreted as a guarantee given by the producer or Assessment Body, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

Characteristic	Assessment of characteristic			
3.1 Mechanical resistance and stability*) (BWR1)				
Characteristic load-carrying capacity	See Annex B			
Stiffness	No performance determined			
Ductility in cyclic testing	No performance determined			
3.2 Safety in case of fire (BWR2)				
Reaction to fire	The post bases are made from steel classified as Euroclass A1 in accordance with EN 13501-1 and Commission Delegated Regulation 2016/364			
3.3 Hygiene, health and the environment (BWR3)				
Influence on air quality	The product does not contain/release dangerous substances specified in TR 034, dated March 2012 0**)			
3.7 Sustainable use of natural resources (BWR7)	No Performance Determined			
3.8 General aspects related to the performance of the product	The post bases have been assessed as having satisfactory durability and serviceability when used in timber structures using the timber species described in Eurocode 5 and subject to the conditions defined by service class 1, 2 and 3			
Identification	See Annex A			

^{*)} See additional information in section 3.9 - 3.11.

^{**)} In addition to the specific clauses relating to dangerous substances contained in this European technical Assessment, there may be other requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Regulation, these requirements need also to be complied with, when and where they apply.

3.9 Mechanical resistance and stability

Safety principles and partial factors

The characteristic load-carrying capacities are based on the characteristic values of the connections with metal fasteners, the steel components and the timber post.

In the case of timber failure or failure of the metal fasteners, the design values shall be calculated according to EN 1995-1-1 by dividing the characteristic values of the load-carrying capacities by different partial factors for the strength properties, and in addition multiplied with the coefficient k_{mod} .

In the case of steel failure, the design value shall be calculated according to EN 1993-1-1 by reducing the characteristic values of the load-carrying capacity with different partial factors.

The design value of the load-carrying capacity is the smaller value of all load-carrying capacities:

$$F_{Rd} = min \left\{ \frac{k_{mod} \cdot F_{Rk,T}}{\gamma_{M,T}}; \frac{F_{Rk,S}}{\gamma_{Mi,S}} \right\}$$

Therefore, for timber failure or failure of the metal fasteners the load duration class and the service class are included. The different partial factors γ_M for steel or timber failure, respectively, are also correctly taken into account

Mechanical resistance and stability

See Annex B for the characteristic load-carrying capacity in the different directions F_1 to F_5 for solid timber of strength class C24 according to EN 338:2016. Using the load-carrying capacities of the post bases, the specifications in Annex A must be fulfilled.

The characteristic capacities of the post bases are determined by a combination of calculation according to Eurocode 3 and Eurocode 5 and testing. They should be used for designs in accordance with Eurocode 3 and Eurocode 5 or a similar national code.

No performance has been determined in relation to ductility of a joint under cyclic testing. The contribution to the performance of structures in seismic zones, therefore, has not been assessed.

No performance has been determined in relation to the joint's stiffness properties - to be used for the analysis of the serviceability limit state.

No performance has been determined in relation to the anchorage of the post bases in the foundation. It must be checked by the designer of the structure to ensure it is not less than the post base capacity and, if necessary, the post base capacity reduced accordingly. Therefore the specifications for the lever arms $e_{F2/F3}$ (for load case F_2 / F_3) and $e_{F4/F5}$ (for load case F_4 / F_5) in annex A have to be considered. The lever arm is the distance between the top edge of the foundation and the load.

3.10 Aspects related to the performance of the product

2.7.1 Corrosion protection in service class 1 and 2. In accordance with ETAG 015 the post bases are produced from steel grade S235JR according to EN 10025-2:2005-04 with a minimum characteristic yield strength of $R_{eH} = 235 \text{ N/mm}^2$ and a minimum characteristic tensile strength of $R_m = 360 \text{ N/mm}^2$ and steel grade DD11 according to EN 10111-2:2008 with a minimum characteristic yield strength of R_{eL} = 170 N/mm². The steel tubes are produced of steel grade S195T according to DIN EN 10255:2007 with a minimum characteristic yield strength of $R_{eH} = 195$ N/mm² and a minimum characteristic tensile strength of $R_m = 320 \text{ N/mm}^2$. The steel rods are produced of steel grade S235JRC+C according to EN 10277-2:2008-06 with a minimum characteristic yield strength of $R_{p0,2} = 260 \text{ N/mm}^2$ and a minimum characteristic tensile strength of $R_m = 390 \text{ N/mm}^2$. The threaded rods correspond to property class 4.8 according to DIN EN ISO 898-1:2009-08

The post bases are for use in timber structures subject to the service classes 1, 2 and 3 of Eurocode 5 and for connections subject to static or quasi-static loading. In service class 1 and 2 the corrosion protection is given by hot-dip zinc coating Z275 according to EN 10147 or zinc coating according to EN1461 with a minimum thickness of 8 μ m. In service class 3 the corrosion protection is given by hot-dip zinc coating Z350 according to EN 10147 or zinc coating according to EN1461 with a minimum thickness of 55 μ m.

The metal fasteners must also have a zinc coating according to EN ISO 2081 corresponding to the relevant service class 1, 2 or 3 of EN 1995-1-1.

3.11 General aspects related to the fitness for use of the product

The performances given in this ETA are based on the following:

- The timber post
 - shall be restrained against rotation, and supported at the lower and upper end
 - shall be strength class C24 or better according

to EN 338:2016

- shall be free from wane in the post base
- must fulfil the requirements regarding minimum dimensions (see Annex A)
- end grain must in general be plane on the base plate or spacer of the post base or at a maximum distance given in Annex A.
- The post base shall be installed centrically in the cross-section of the timber column.
- The actual end bearing capacity of the timber member to be used in conjunction with the post base is checked by the designer of the structure to ensure it is not less than the post base capacity and, if necessary, the post base capacity reduced accordingly.
- There are no specific requirements relating to preparation of the timber members.
- The minimum insertion depth in the turnbuckles should be the diameter of the rod.
- The base plates of the post bases with steel tubes as support must in general be plane on the tube's end.
- The anchorage of the post base in the foundation is not part of this ETA. It must be checked by the designer of the structure to ensure it is not less than the post base capacity and, if necessary, the post base capacity reduced accordingly. Therefore, the specifications for the lever arms e_{F2/F3} (for load case F₂ / F₃) and e_{F4/F5} (for load case F₄ / F₅) in Annex A have to be considered. The lever arm is the distance between the top edge of the foundation and the load.

4 Attestation and verification of constancy of performance (AVCP)

4.1 AVCP system

According to the decision 97/638/EC of the European Commission1, as amended, the system(s) of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) is 2+.

5 Technical details necessary for the implementation of the AVCP system, as foreseen in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at ETA-Danmark prior to CE marking.

Issued in Copenhagen on 2017-04-24 by

Thomas Bruun Managing Director, ETA-Danmark

Annex A Product details and definitions

Table A.1 Specifications of the post bases

Post base		Metal Fas	teners**	Post [mm]	Distan	ices [mn	n]
Type	Drawing number	Nails/Screws*	Dowels/Bolts	min b/h	max. a	e _{F2/F3}	e _{F4/F5}
A001	BB00 0003001 A	4 x F 12x120mm	-	140/140	110	110	110
A002	540 23 0001	2 x P 10x80mm	-	80 to 160 /140	163	-	-
A002	340 23 0001	4 x P 6x80mm	-	80 to 160 /140	163	-	-
A003	540 23 0002	-	2 x D Ø10 or 2 x Bo Ø10	100/130	163	251	176
A004	541 23 0001	-	1 x Bo M10	120/120	163	-	263
H001	555 23 0000	4 x F 12x120mm	-	130/130	135 to 200	a	a
H002	555 23 0001	4 Nails 4x40mm	-	70 to 170 /116	139 to 204	-	ı
H003	555 23 0002	4x F 12x120mm	1 x D Ø10 or 1 x Bo M10	130/130	135 to 200	a + 60	a
H004	555 23 0003	-	2 x D Ø10 or 2 x Bo Ø10	100/130	135 to 200	a +13	a +100
H100	550 13 0004	4x F 12x120mm	-	130/130	140 to 205	a	a
BRN M20	BB00 0004001 A	4x F 10x100mm	-	120/120	40 to 146	a	a
ASH	BB00-0023-003 BB00-0023-004 BB00-0023-005 BB00-0023-006 BB00-0023-007 BB00-0023-009 BB00-0023-010	2 x F 10x50mm	-	71/71 81/81 91/91 101/101 121/121 141/141 161/161 201/201	10	103	70
ASH r	BB00-0023-011 BB00-0023-012 BB00-0023-013 BB00-0023-014	2 x F 10x50mm	-	Ø80 Ø100 Ø120 Ø140	10	113	60
ASH g	BB00-0049-001	4 x F 10x50mm	-	85/85	10	122	122
ASH g	BB00-0049-002	8 x F 10x50mm	-	105/105	10	139	139
ASH z	BB00-0045-001 BB00-0045-002 BB00-0045-003 BB00-0045-004 BB00-0045-005	2 x F 10x50mm	-	101/85 121/114 141/114 161/114 201/140	12 12 12 14 14	163 163 163 147 137	65 71 85 88 88

^{*} P = partial thread; F = full thread

^{**} The position of the fasteners shall always be at maximum distance to the end grain of the post.

Continuation of Table A.1 Specifications of the post bases

]	Post base	Metal Fast	Post [mm]	Distar	Distances [mm]		
Type	Drawing number	Screws*	Dowels/Bolts	min b/h	max. a	e _{F2/F3}	e F4/F5
PFTR S	525 32 0000 525 32 0001 525 32 0002 525 32 0003 525 32 0004 525 32 0005	2 x F 10x50mm	-	71/112 81/112 91/112 101/112 121/112 141/112	58	130	58
PFTR a	520 32 0000 520 32 0001 520 32 0002 520 32 0003 520 32 0004 520 32 0005	4 x F 10x50mm	1	71/80 81/80 91/80 101/80 121/80 141/80	16	386	51
PFTR 200 U	BB00-0048-001 BB00-0048-002 BB00-0048-003 BB00-0048-004 BB00-0048-005 BB00-0048-006	2 x F 10x50mm	-	71/115 81/115 91/115 101/115 121/115 141/115	50	140	73
PFTR 250 U	BB00-0048-007 BB00-0048-008 BB00-0048-009 BB00-0048-010 BB00-0048-011 BB00-0048-012	2 x F 10x50mm	-	71/115 81/115 91/115 101/115 121/115 141/115	100	190	114
PFTR hv	BB00-0035-009 BB00-0035-013	2 x F 12x120mm	-	140/140 130/130	122 to 180	a	a

^{*} P = partial thread; F = full thread

Table A.2 Specifications of the metal fasteners according to EN 14592

Fastener type		Size (mm)		Material	Finish
	Diameter	Length	Threaded length		
Dowels	10 mm			S235	Galvanic zinc coating
Bolts	10 mm			4.6	Galvanic zinc coating
Nails	4 mm	min 40 mm	-	$f_{u,k} {\geq 600~N/mm^2}$	Galvanic zinc coating
Screws	6 mm	min 80 mm	min 48 mm	$f_{u,k}\!\geq 360\;N/mm^2$	Galvanic zinc coating
Screws	10 mm	min 50 mm	min 35 mm	$f_{u,k}\!\geq 360\;N/mm^2$	Galvanic zinc coating
Screws	10 mm	min 80 mm	min 48 mm	$f_{u,k}\!\geq 360\;N/mm^2$	Galvanic zinc coating
Screws	10 mm	min 100 mm	min 88 mm	$f_{u,k}\!\geq 360\;N/mm^2$	Galvanic zinc coating
Screws	12 mm	min 120 mm	min 105 mm	$f_{u,k}\!\geq 360\;N/mm^2$	Galvanic zinc coating

The load-carrying-capacities of the metal fasteners were calculated according to Eurocode 5 for lateral loads. The contribution to the load-carrying capacity due to the rope effect was considered according to Eurocode 5.

^{**} The position of the fasteners shall always be at maximum distance to the end grain of the post.

Annex B Characteristic load-carrying capacities

Table B.1 Characteristic load-carrying capacities for post bases in kN

	Post Base		F ₁ (Compression)		F ₁ (Tension)		F_2/F_3		F ₄ /F ₅	
Type	Metal Fasteners	Timber	St	eel	Timber	Steel	Timber	Steel	Timber	Steel
A001	4 x F 12x120 mm	100,0	100,0	87,8	18,7	8,5	12,0	3,0	12,0	3,0
A001		γ _M (T)	γм,о	γ _{M,1}	γм (с)	үм,о	γM (C)	γм,о	γм (с)	γм,о
	2 x P 10x80 mm	26,3	32,5	-	9,2	2,0	-	-	-	-
A002	4 x P 6x80 mm	26,3	32,5	-	9,2	2,0	-	-	-	-
		γ _M (T)	γм,о	-	γм (с)	үм,о	-	-	-	-
A003	2 x D Ø10 or 2 x Bo M10	100,7	-	87,8	25,1	7,3	13,1	0,9	2,5	1,7
		γ _M (T)	-	$\gamma_{\rm M,1}$	γм (с)	үм,о	γм (т)	үм,о	γм (с)	үм,о
	1 x D Ø10	91,5	91,5	87,8	6,4	7,3	-	-	5,1	0,9
A004	1 x Bo M10	91,5	91,5	87,8	7,6	7,3	-	-	6,4	0,9
		γ _M (T)	$\gamma_{\mathrm{M,o}}$	$\gamma_{\rm M,1}$	γ _M (J)	үм,о	-	-	γм (С)	үм,о
H001	4 x F 12x120 mm	55,9	55,9	57,9	18,7	6,1	12,0	0,8	6,0	1,3
H001		γ _M (T)	үм,о	$\gamma_{\rm M,1}$	γм (С)	үм,о	γм (С)	үм,о	γм (с)	үм,о
H002	4 x N 4x40mm	32,6	30,6	ı	5,5	0,9	-	-	-	-
11002		γ _M (T)	$\gamma_{\mathrm{M,o}}$	ı	γм (С)	үм,о	-	-	-	ı
	4 x F 12x120 mm	44,8	44,8	57,9	18,7	6,1	12,0	0,8	6,0	1,3
	1 x D Ø10	44,8	44,8	57,9	6,4	6,1	5,1	0,5	-	ı
H003	1 x Bo M10	44,8	44,8	57,9	7,6	6,1	6,4	0,5	-	1
		γ _M (T)	үм,о	$\gamma_{\mathrm{M,1}}$	γм (с)	үм,о	γ _{M (C)}	үм,о	-	-
H004	2 x D Ø10 or 2 x Bo M10	61,6	61,6	57,9	25,1	6,1	2,5	0,7	13,1	0,9
		γ _M (T)	γ _{M,1}	$\gamma_{\mathrm{M,1}}$	γм (с)	үм,о	γм (т)	үм,о	γм (т)	үм,о
H100	4 x F 12x120 mm	55,9	55,9	57,3	18,7	6,1	12,0	0,8	12,0	1,3
11100		γ _M (T)	үм,о	γ _{M,1}	γм (с)	үм,о	γм (С)	үм,о	γм (с)	үм,о
BRN	4 x F 10x100 mm	84,5	ı	64,2	14,8	10,0	9,5	1,7	9,5	1,7
M20		γ _M (T)	-	$\gamma_{\mathrm{M,1}}$	γм (с)	үм,о	γM (C)	үм,о	γ _M (C)	үм,о

 $\gamma_{M\,(T)}\!=\!$ partial factor for solid timber according to EN 1995-1-1 and national annex $\gamma_{M\,(C)}\!=\!$ partial factor for connections according to EN 1995-1-1 and national annex $\gamma_{M,o;}\,\gamma_{M,1}\!=\!$ partial factor according to EN 1993-1-1 and national annex

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Continuation of Table B.1 Characteristic load-carrying capacities for post bases in kN

Post B	ase	F ₁ (Com	pression)	F ₁ (Te	nsion)	F ₂ /F ₃		F ₄ /F ₅	
Туре	Size	Timber	Steel	Timber	Steel	Timber Steel		Timber	Steel
	71	7,56	37,5	7,56	7,03	7,61	6,77	7,61	6,77
	81	7,56	37,5	7,56	8,81	10,1	10,3	10,1	10,3
	91	7,56	37,5	7,56	13,3	13,0	10,3	13,0	10,3
	101	7,56	37,5	7,56	13,3	15,5	10,3	15,5	10,3
ASH	121	7,56	37,5	7,56	13,3	15,5	10,3	15,5	10,3
	141	7,56	37,5	7,56	13,3	15,5	10,3	15,5	10,3
	161	7,56	37,5	7,56	13,3	15,5	10,3	15,5	10,3
	201	7,56	37,5	7,56	13,3	18,7	12,5	18,7	12,5
		γм (С)	γм,2	γ _M (C)	үм,о	γм (С)	γм,ο	γм (т)	γм,о
	Ø 80	7,56	37,5	7,56	4,54	7,59	3,20	7,59	3,20
	Ø 100	7,56	37,5	7,56	6,40	12,3	5,64	12,3	5,64
ASH r	Ø 120	7,56	37,5	7,56	5,26	18,1	5,56	18,1	5,56
	Ø 140	7,56	37,5	7,56	6,29	24,9	7,78	24,9	7,78
		γ _M (C)	$\gamma_{\mathrm{M,2}}$	γ _M (C)	үм,о	γм (т)	үм,о	γ _M (T)	$\gamma_{\mathrm{M,o}}$
	85	15,1	38,9	15,1	12,4	5,04	4,23	5,04	4,23
ASH g	105	22,8	59,0	22,8	10,0	10,1	4,74	10,1	4,74
		γм (С)	γ _{M,2}	γM (C)	γм,о	γм (С)	үм,о	γм (С)	үм,о
	101	21,0	32,2	7,56	5,97	10,8	3,32	6,74	3,89
	121	29,4	45,0	7,56	6,93	10,8	4,61	7,01	3,73
ASH z	141	37,8	57,9	7,56	3,85	10,8	2,98	7,31	4,74
АЗП Z	161	50,4	77,2	7,32	18,6	17,3	16,4	11,3	8,69
	201	63,0	96,5	7,32	18,6	17,3	20,5	11,7	9,85
		γ _M (T)	$\gamma_{\mathrm{M,o}}$	γ _M (C)	γм,о	γм (т)	үм,о	γ _M (T)	үм,о
	71	7,76	13,3	7,36	7,99	5,41	2,96	3,38	3,26
	81	7,76	13,3	7,36	6,74	5,41	2,71	3,38	3,26
	91	7,76	13,3	7,36	5,83	5,41	2,35	3,38	3,26
PFTR S	101	7,76	13,3	7,36	5,13	5,41	2,07	3,38	3,26
	121	7,76	13,3	7,36	4,15	5,41	1,67	3,38	3,26
	141	7,76	13,3	7,36	3,48	5,41	1,40	3,38	3,26
Yaram: partial fa		γ _M (C)	γм,о	γм (с)	γм,ο	γ _M (C)	үм,о	γм (т)	үм,о

 $\gamma_{M(T)}$: partial factor for solid timber according to EN 1995-1-1 and national annex $\gamma_{M(C)}$: partial factor for connections according to EN 1995-1-1 and national annex

 $\gamma_{M,0}$; $\gamma_{M,1}$; $\gamma_{M,2}$: partial factor according to EN 1993-1-1 and national annex

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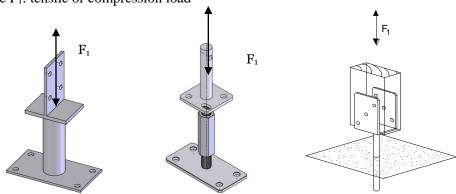
Continuation of Table B.1 Characteristic load-carrying capacities for post bases in kN

Post Base		F ₁ (Com	pression)	F ₁ (Tension)		F_2	/F ₃	F ₄ /F ₅	
Type	Size	Timber	Steel	Timber	Steel	Timber	Steel	Timber	Steel
	71	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
	81	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
	91	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
PFTR a	101	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
	121	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
	141	14,6	51,4	7,32	82,9	4,88	6,32	3,55	7,10
		γм (С)	γM,1	γM (C)	γM,2	γм (С)	γм,о	γм (т)	үм,о
	71	45,9	45,9	7,32	5,59	4,88	2,85	4,70	6,33
	81	45,9	45,9	7,32	4,57	4,88	2,85	4,70	6,33
	91	45,9	45,9	7,32	3,86	4,88	2,85	4,70	6,33
PFTR 200 U	101	45,9	45,9	7,32	3,35	4,88	2,85	4,70	6,33
	121	45,9	45,9	7,32	2,64	4,88	2,85	4,70	6,33
	141	45,9	45,9	7,32	2,18	4,88	2,85	4,70	6,33
		γм (т)	$\gamma_{ m M,o}$	γ _M (C)	γм,о	γм (С)	γм,о	γм (С)	$\gamma_{\rm M,o}$
	71	45,9	45,9	7,32	5,59	4,88	2,18	4,56	4,04
	81	45,9	45,9	7,32	4,57	4,88	2,18	4,56	4,04
	91	45,9	45,9	7,32	3,86	4,88	2,18	4,56	4,04
PFTR 250 U	101	45,9	45,9	7,32	3,35	4,88	2,18	4,56	4,04
	121	45,9	45,9	7,32	2,64	4,88	2,18	4,56	4,04
	141	45,9	45,9	7,32	2,18	4,88	2,18	4,56	4,04
		γм (т)	γм,о	γM (C)	γм,о	γм (С)	γм,о	γм (С)	$\gamma_{\mathrm{M,o}}$
	80	53,5	53,5	18,7	21,1	12,0	0,92	12,0	0,92
PFTR hv	100	53,5	53,5	18,7	7,53	12,0	0,92	12,0	0,92
w partial fa		γм (т	γм,ο	γ _M (C)	γ _{M,0}	γ _M (C)	$\gamma_{\rm M,o}$	γм (С)	γм,о

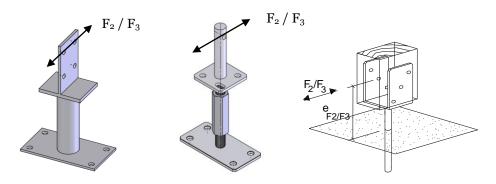
 $\gamma_{M\,(T)}$: partial factor for solid timber according to EN 1995-1-1 and national annex $\gamma_{M\,(C)}$: partial factor for connections according to EN 1995-1-1 and national annex $\gamma_{M,0}$; $\gamma_{M,1}$; $\gamma_{M,2}$: partial factors according to EN 1993-1-1 and national annex

Definitions of forces, their directions and eccentricity

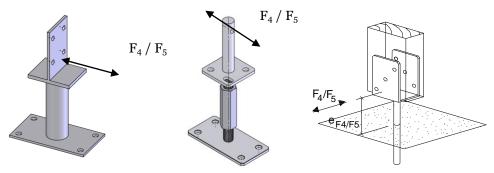
• Force F₁: tensile or compression load



• Force F₂ / F₃: horizontal parallel to the ground plate of the post base and perpendicular to the bolts or dowels



 $\bullet \quad \text{Force } F_4 \, / \, F_5\text{: horizontal load parallel to the ground plate of the post base and parallel to the bolts or dowels} \\$



Acting forces

F₁ axial force (tension or compression) acting along the central axis of the joint

 F_2 and F_3 horizontal force perpendicular to the ground plate of the post base acting with the lever

arm $e_{F2/F3}$ above the foundation

 F_4 and F_5 horizontal force parallel to the ground plate of the post base acting with the lever arm

e_{F4/F5} above the foundation

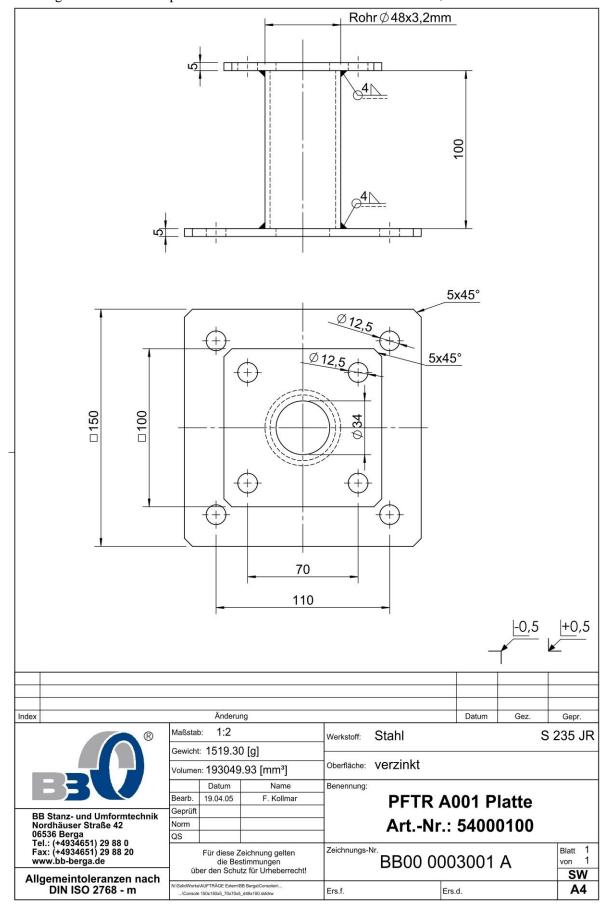
Combined forces

If the forces F_1 and F_2/F_3 or F_4/F_5 act at the same time, the following inequality shall be fulfilled:

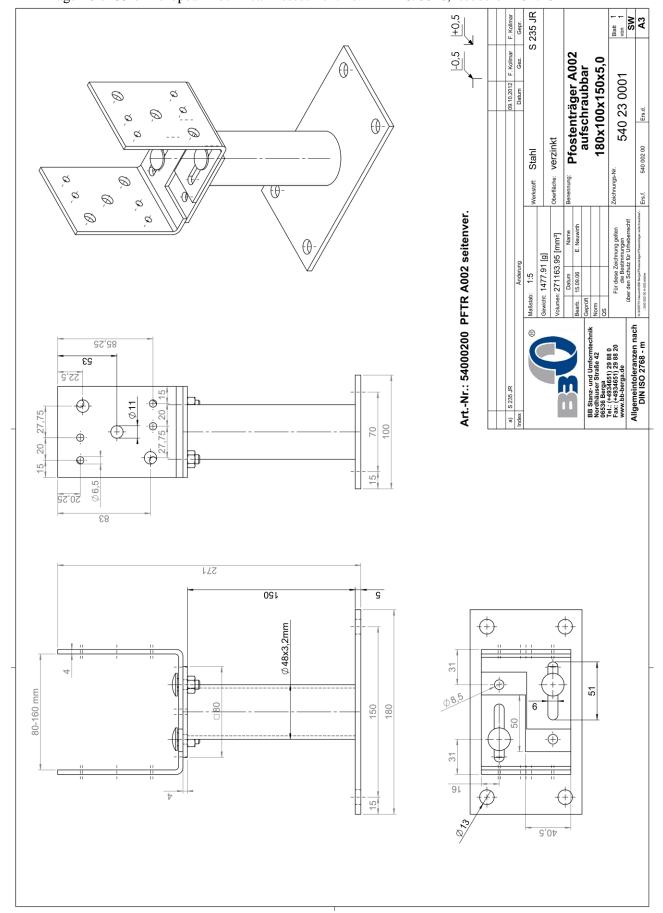
$$\sum \frac{F_{i,Ed}}{F_{i,Rd}} \leq 1$$

The forces F_2 and F_3 or F_4 and F_5 are forces with opposite direction. Therefore only one force F_2 or F_3 , and F_4 or F_5 , respectively, is able to act simultaneously with F_1 .

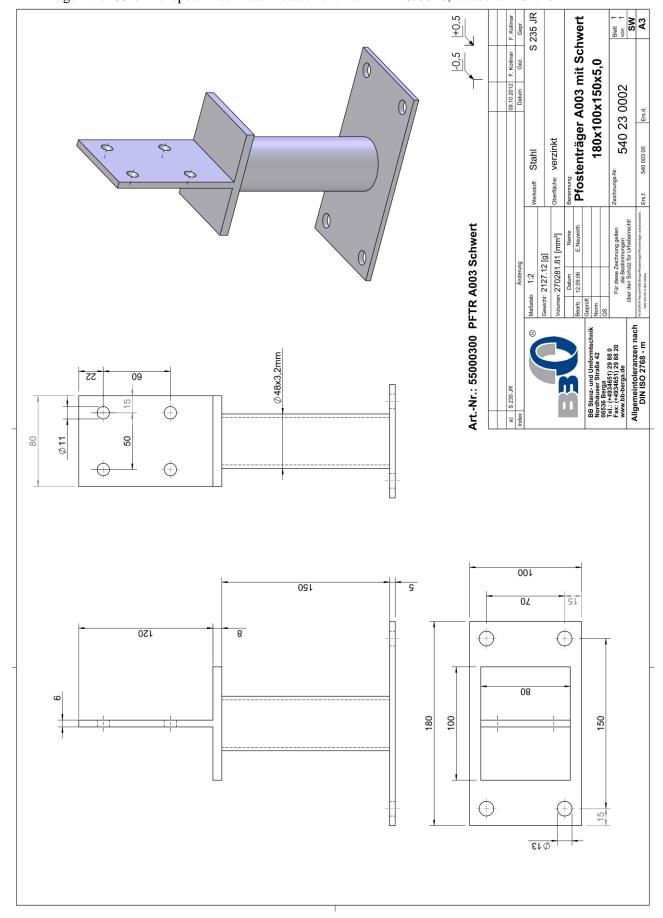
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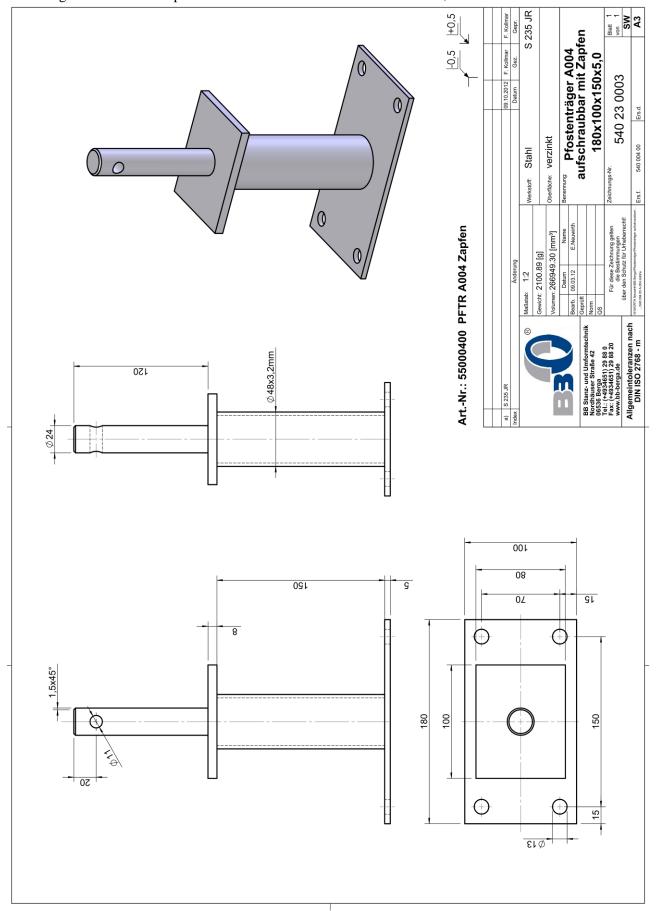
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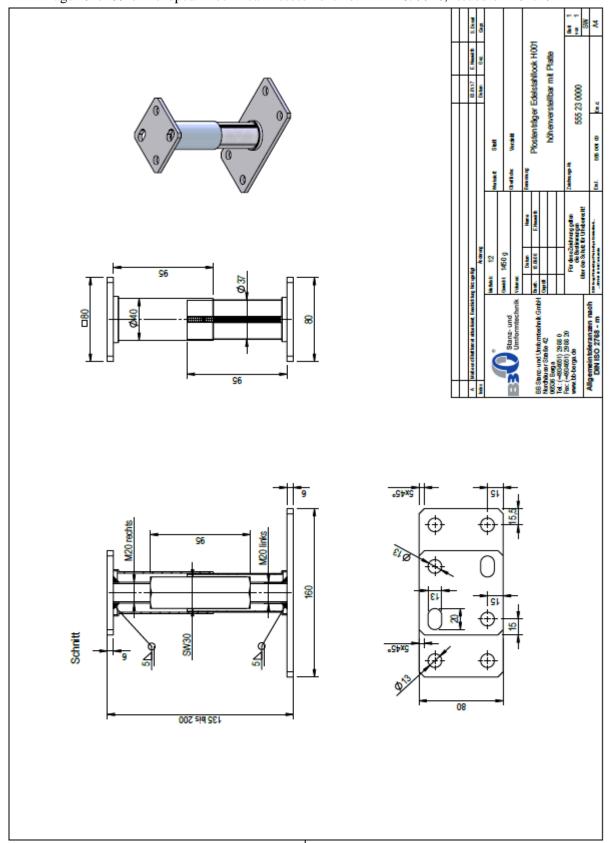
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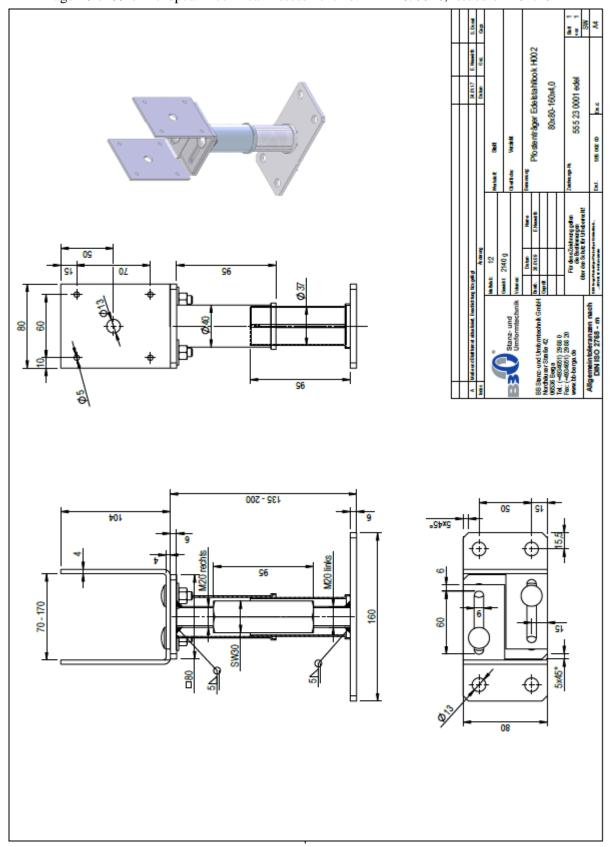
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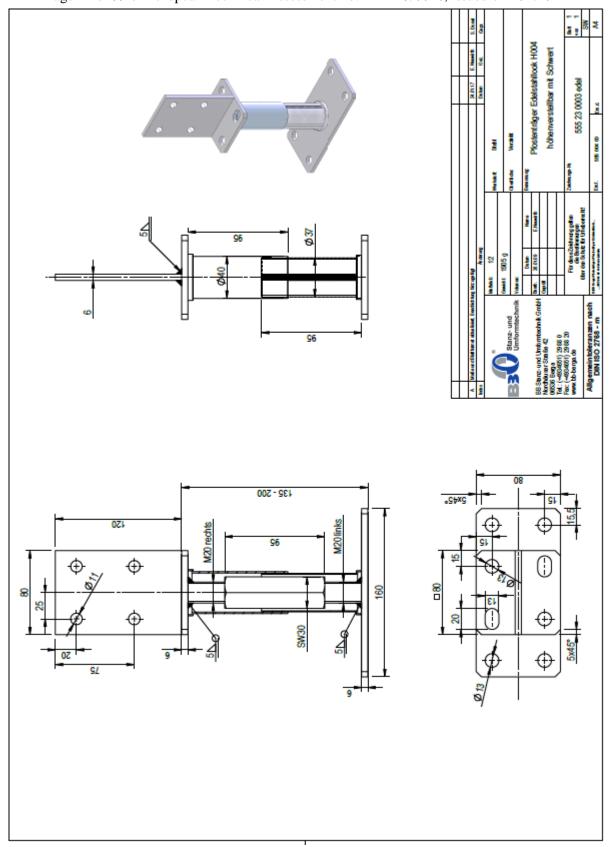


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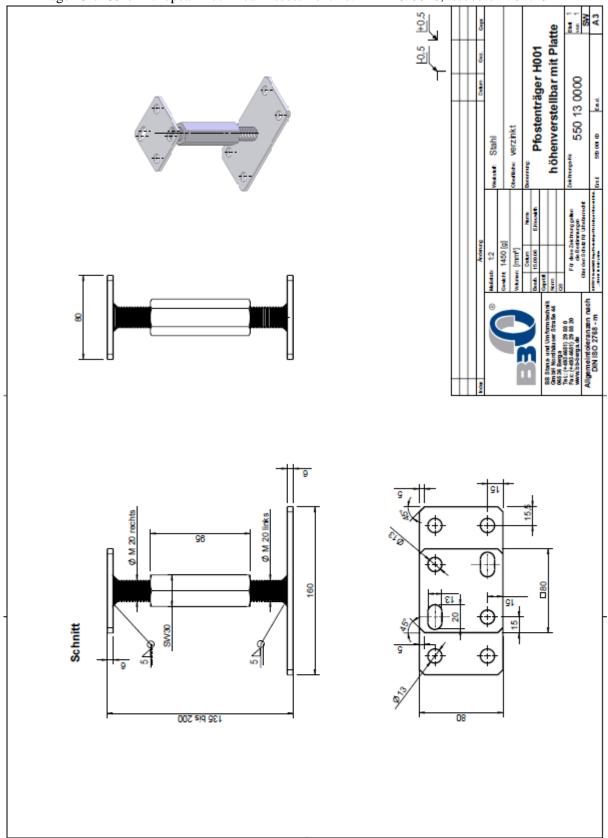


Page 21 of 59 of European Technical Assessment no. ETA-13/0026, issued on 2017-04-24 Piostenträger Edelstahtlock H003 höherverstelbar mit Zapfen 555 23 0002 edel 96 132 - 500 M20 rechts M20 links 150 Ф

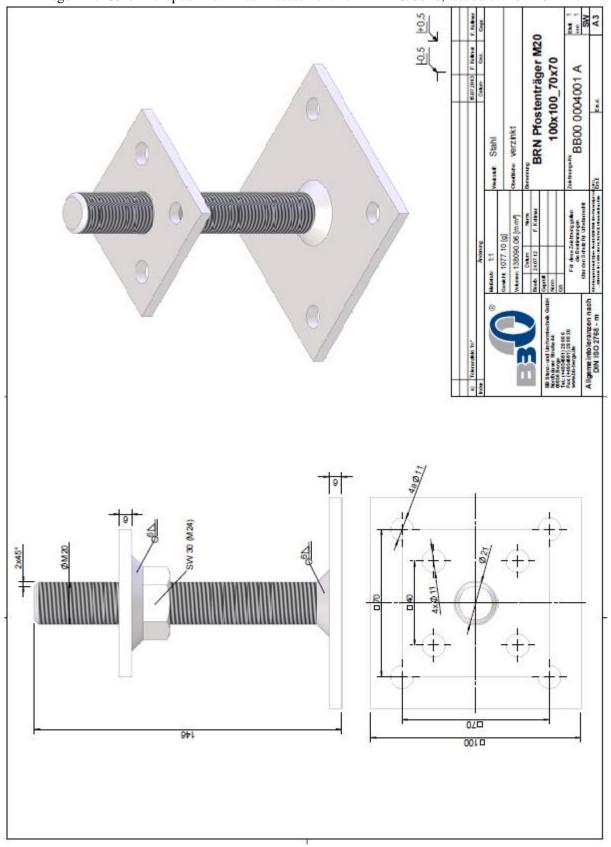
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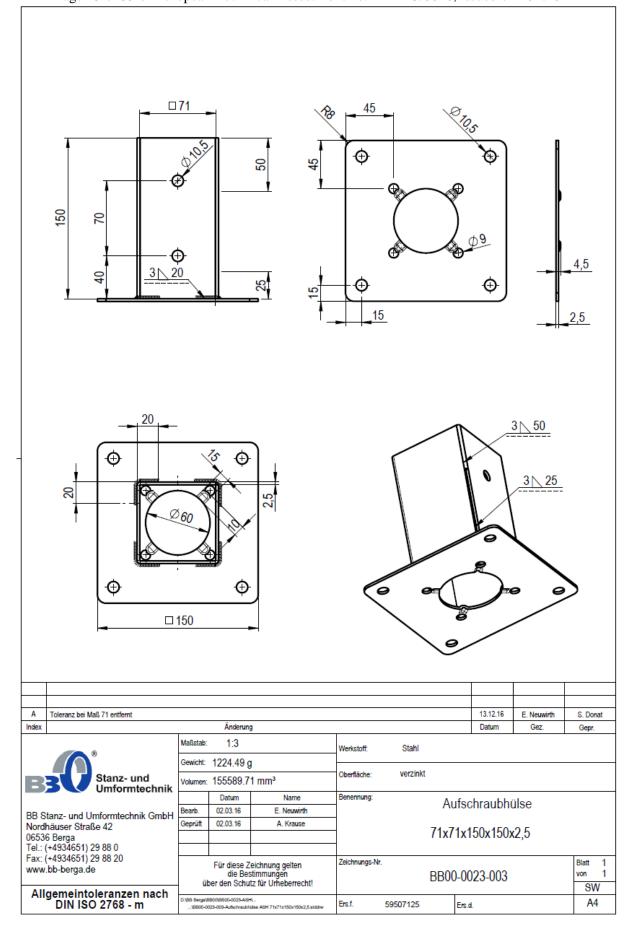


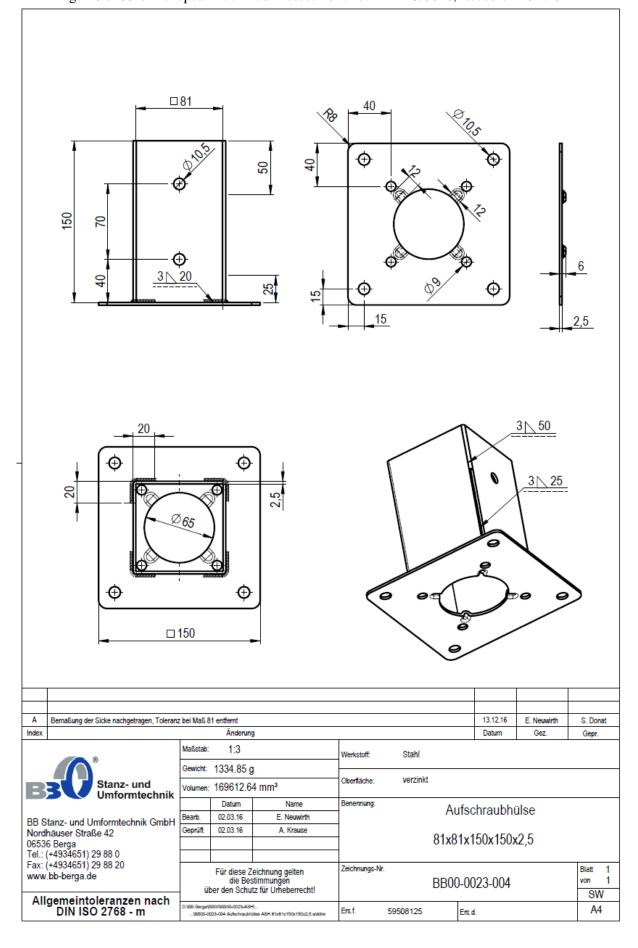
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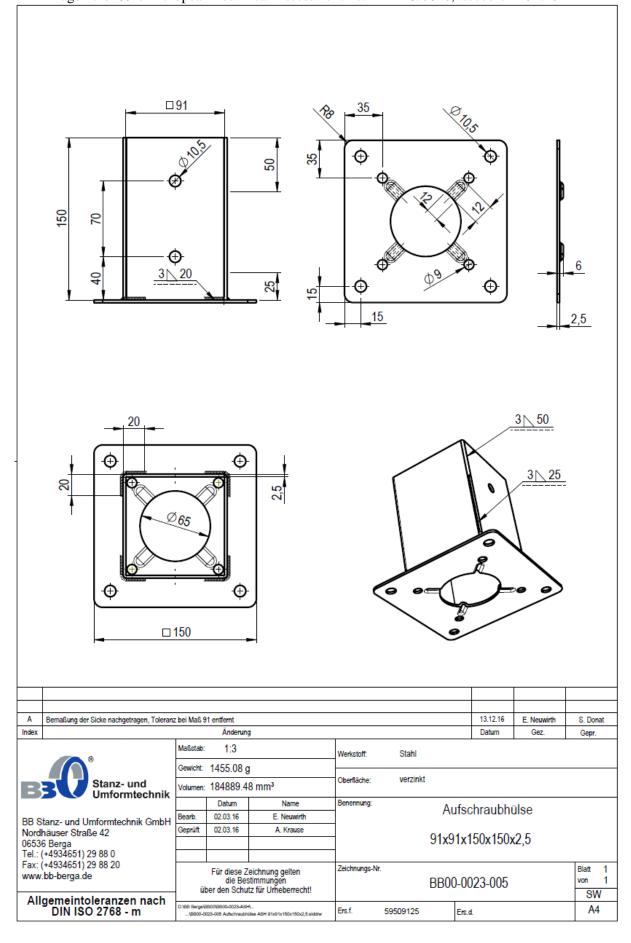


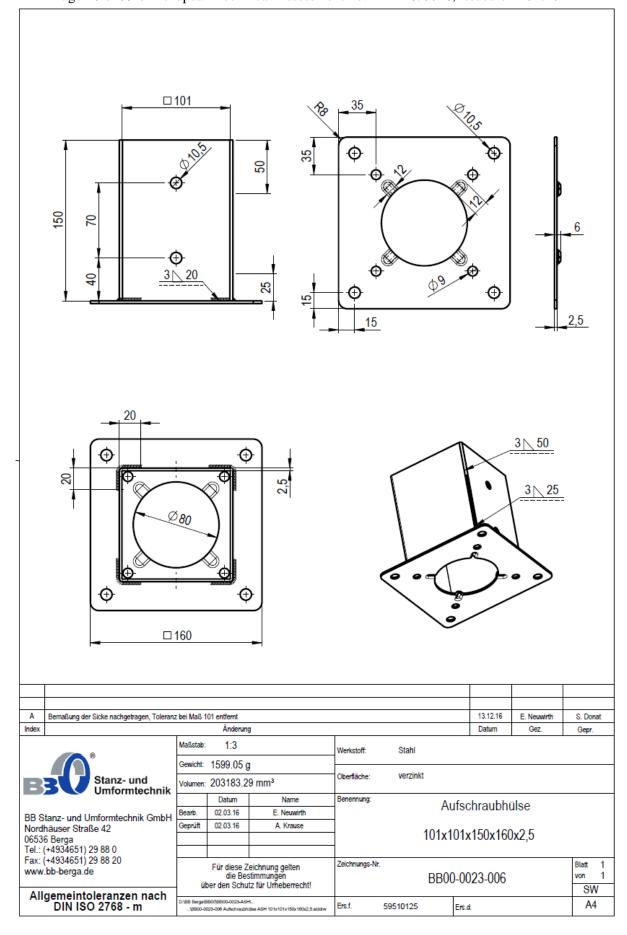
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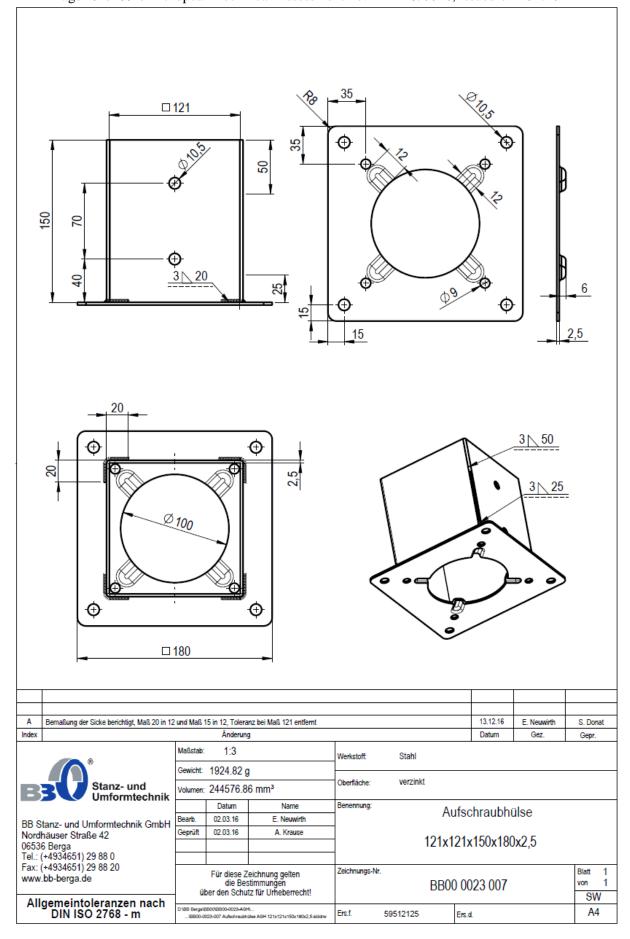


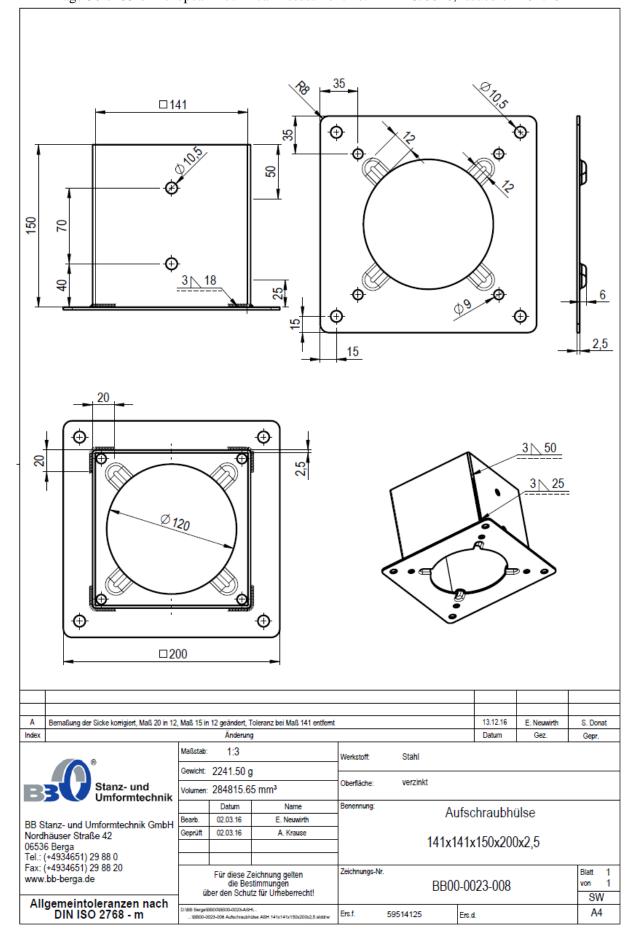


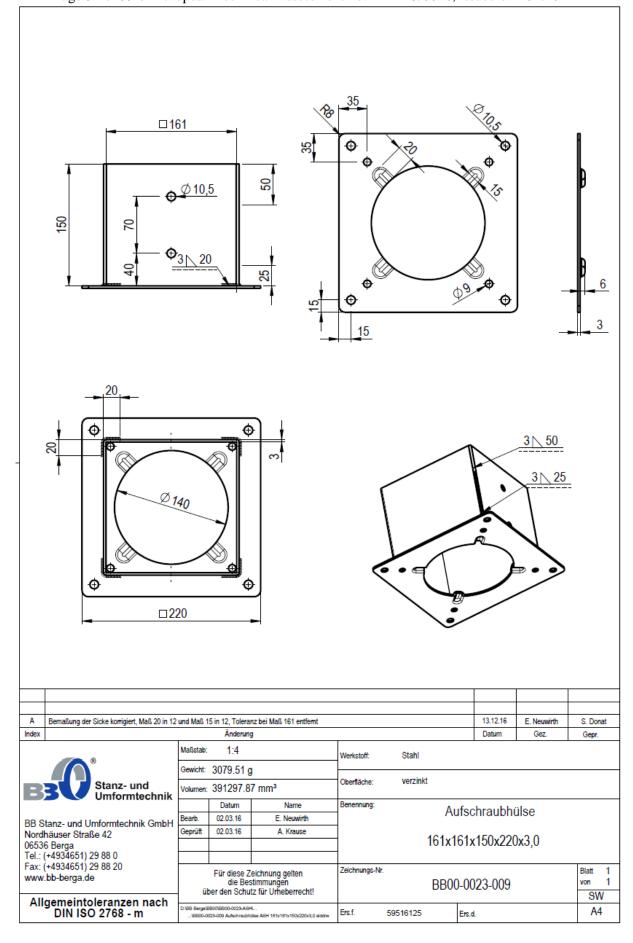


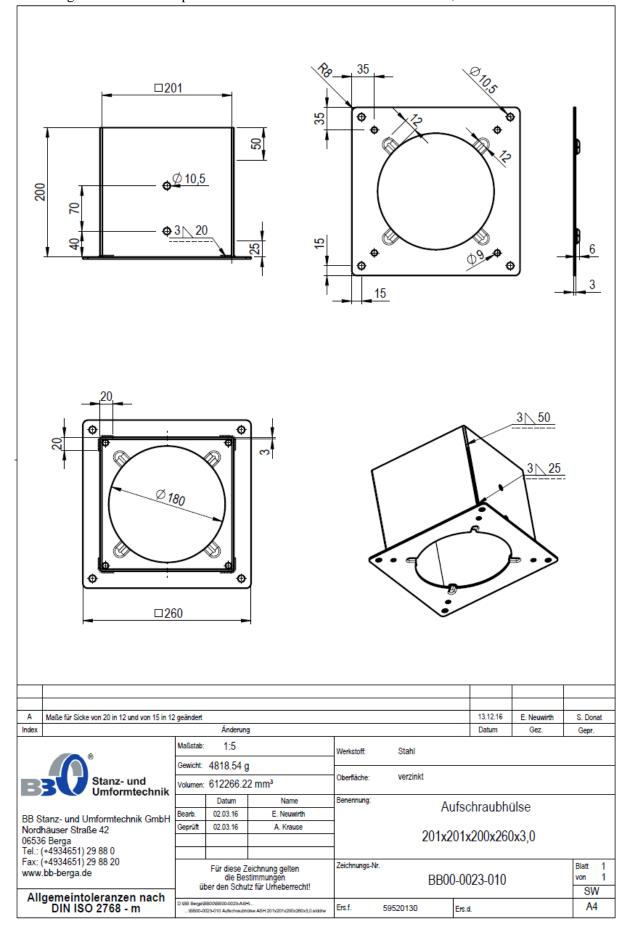


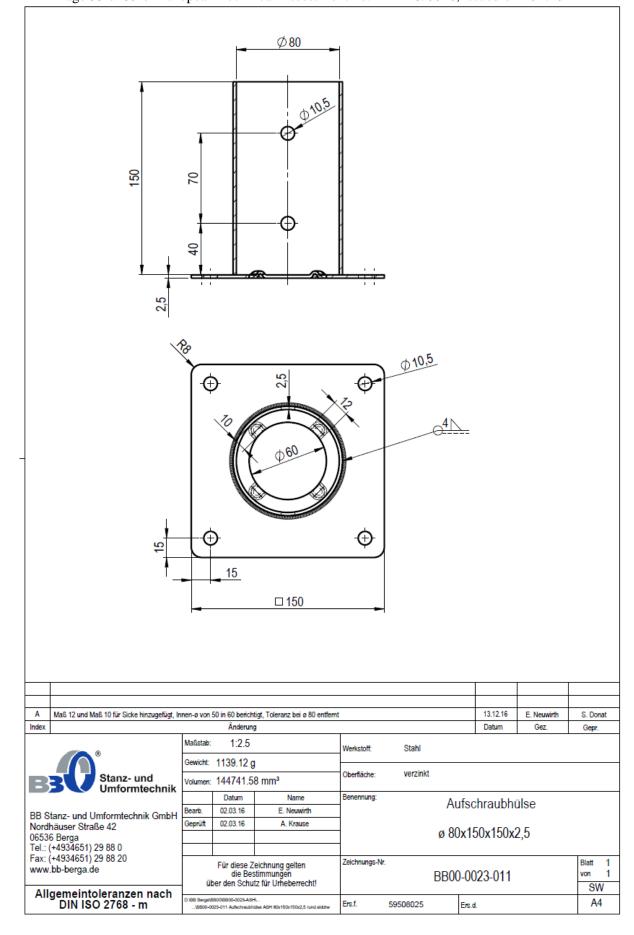


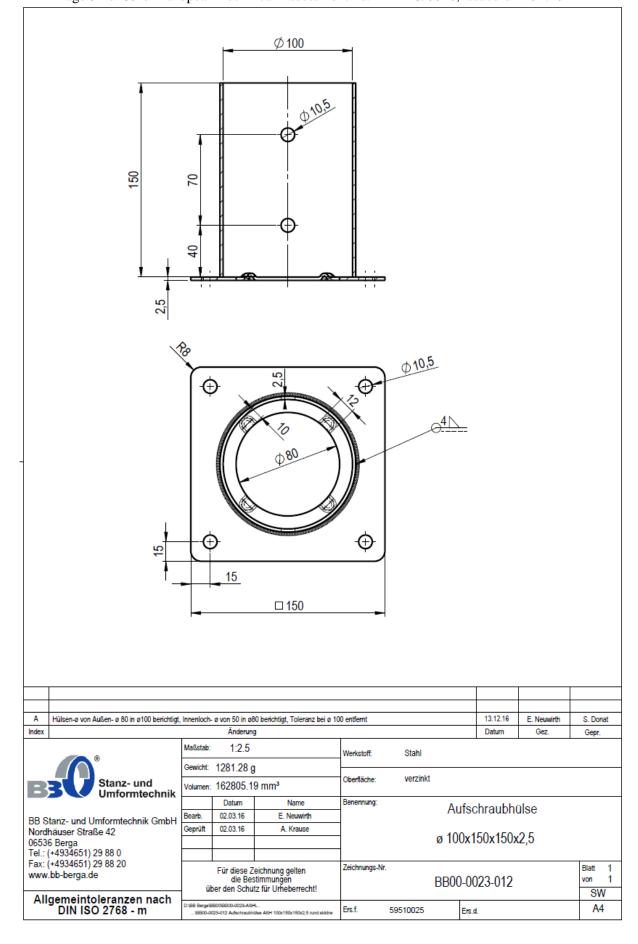


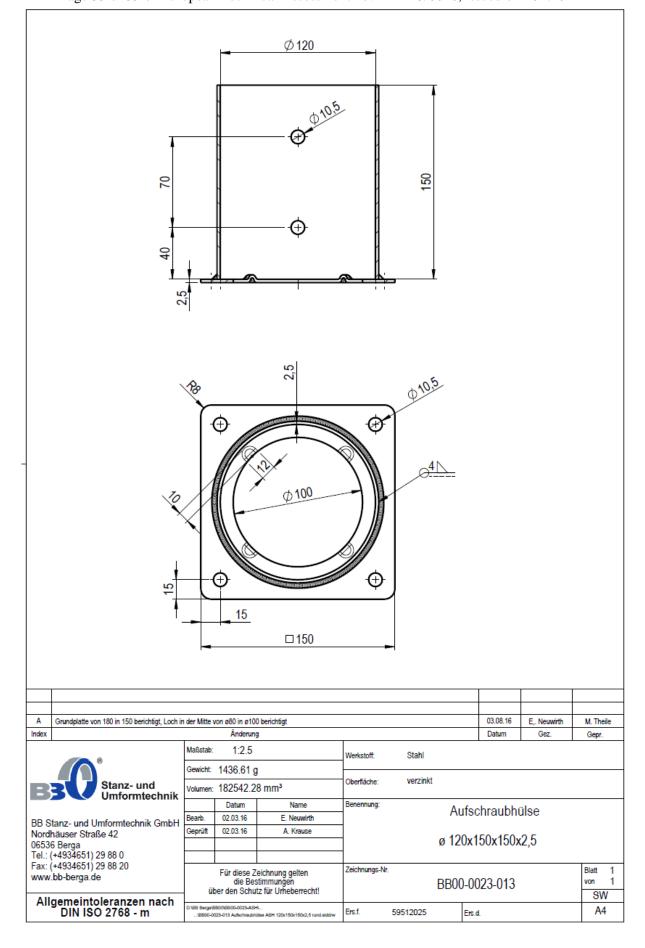


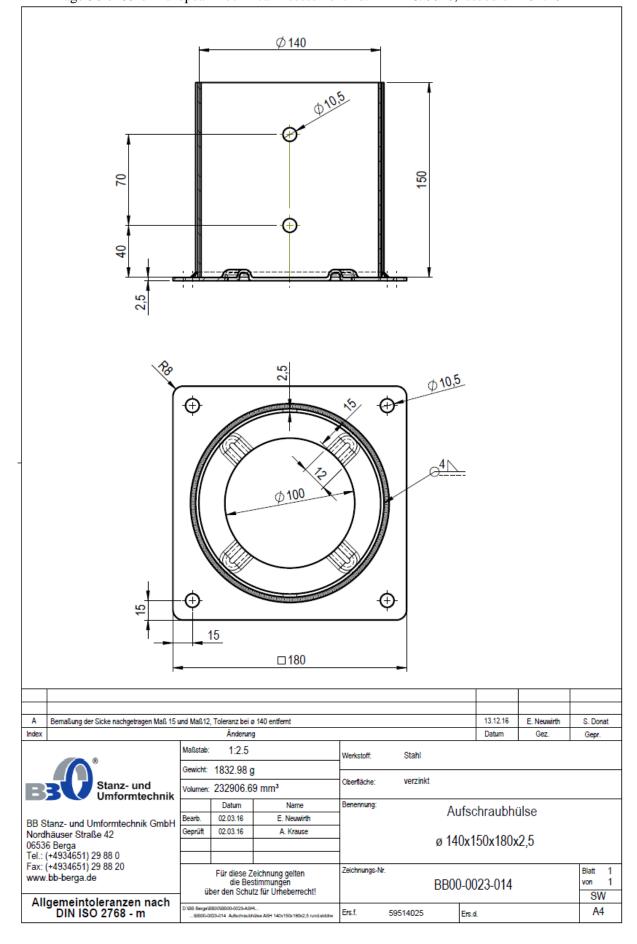




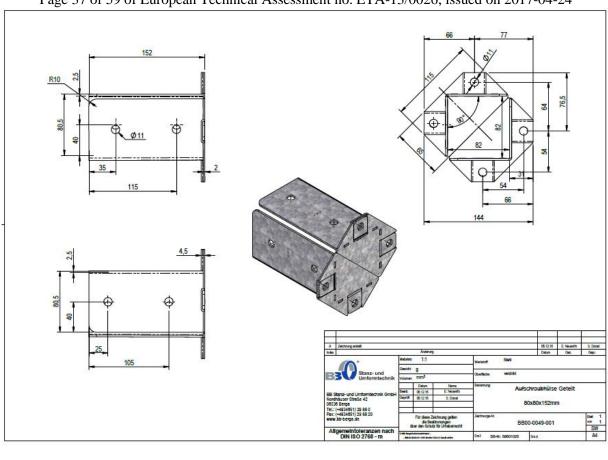


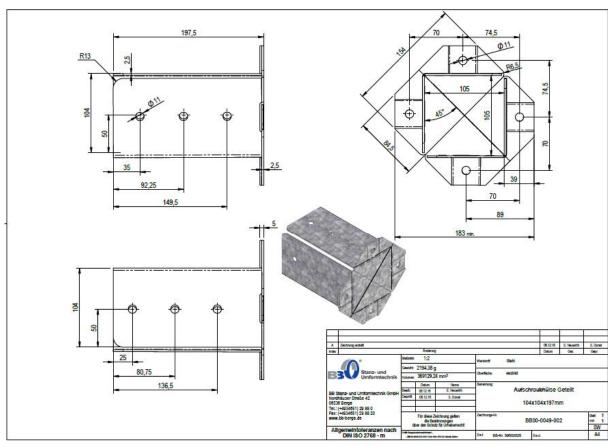


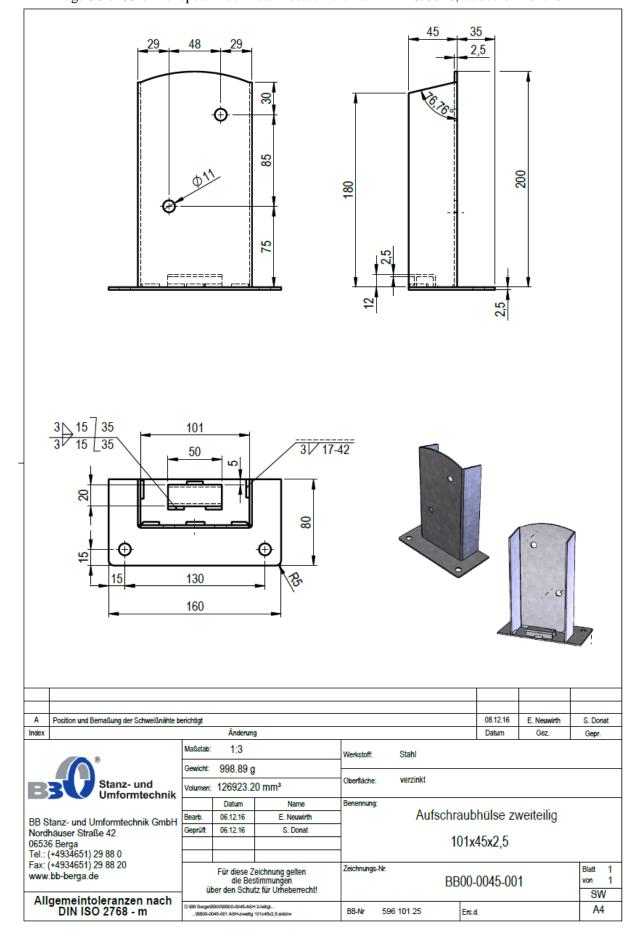


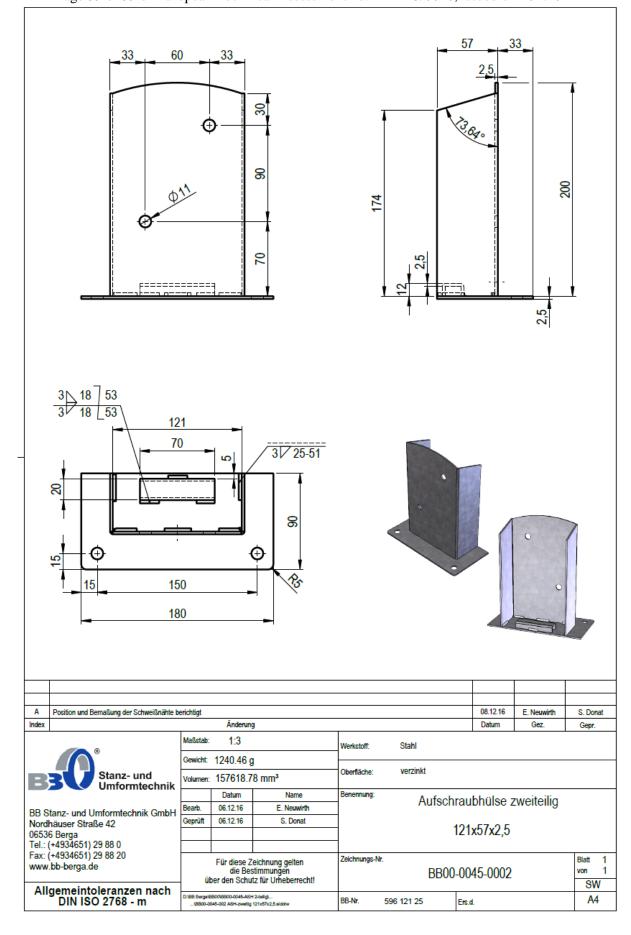


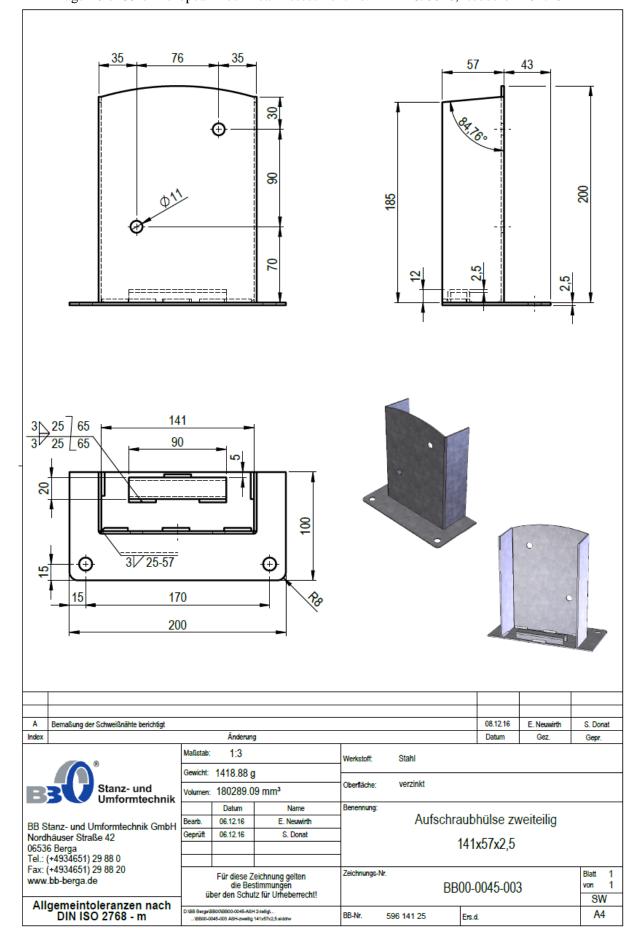
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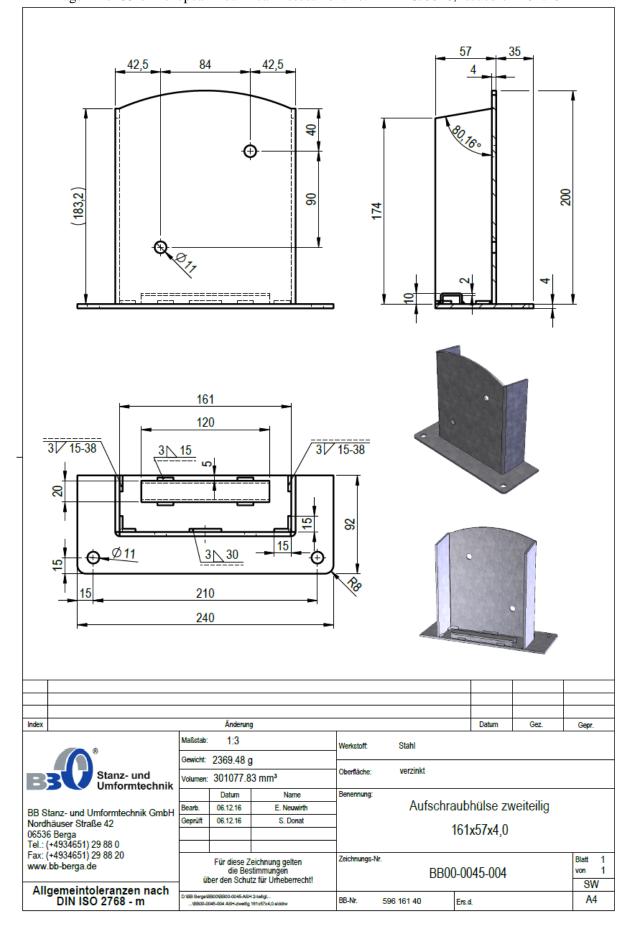


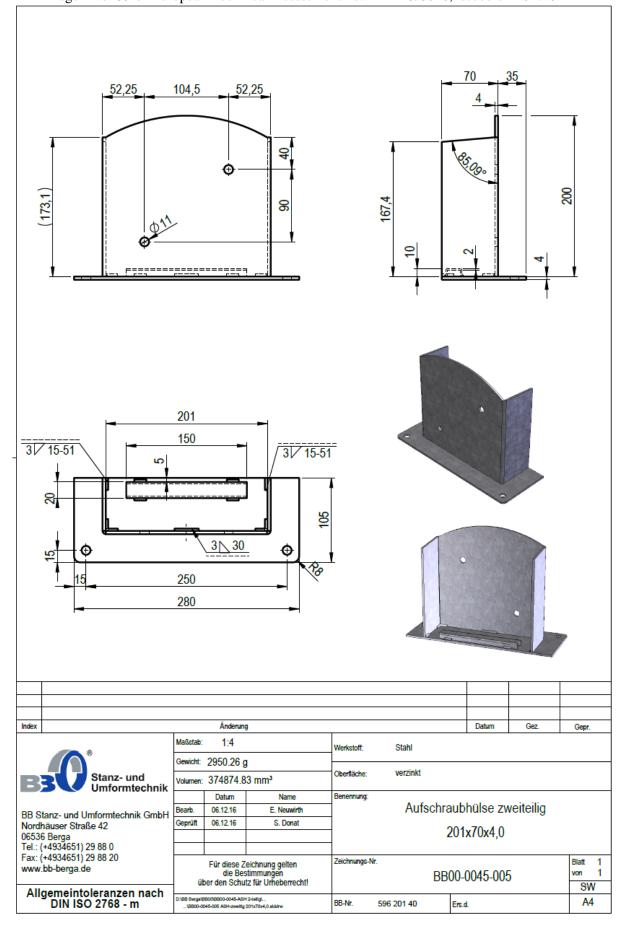


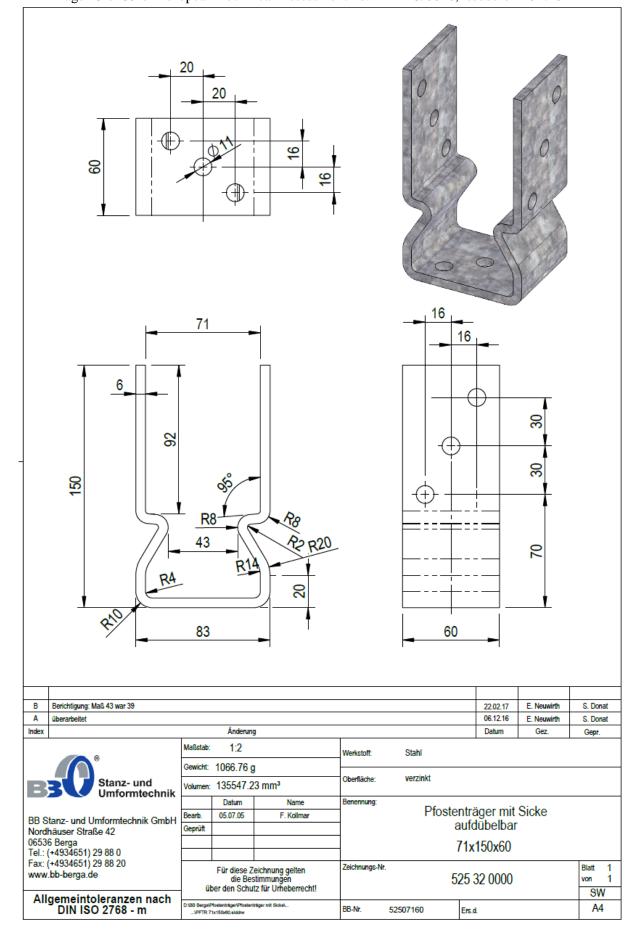


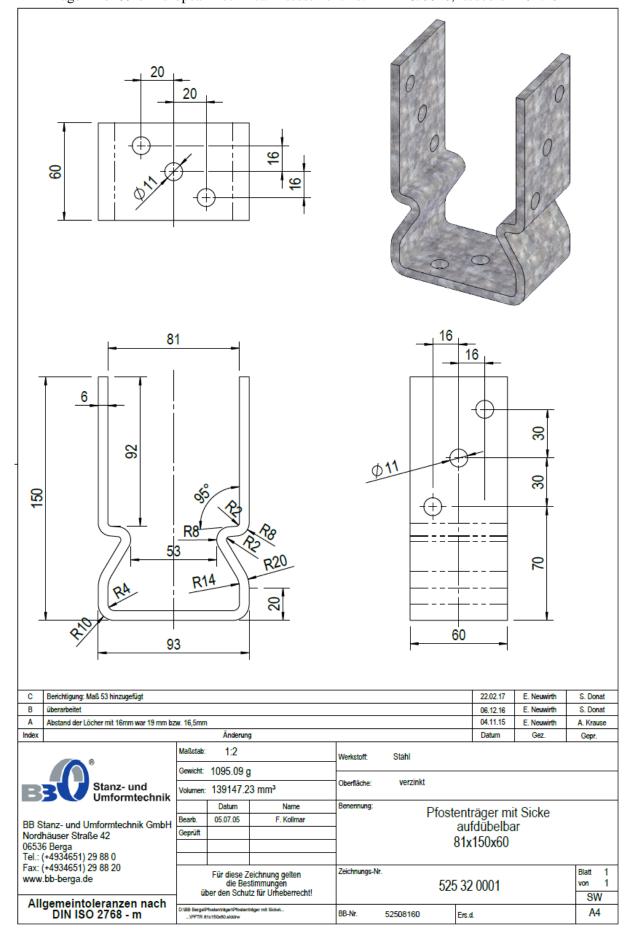


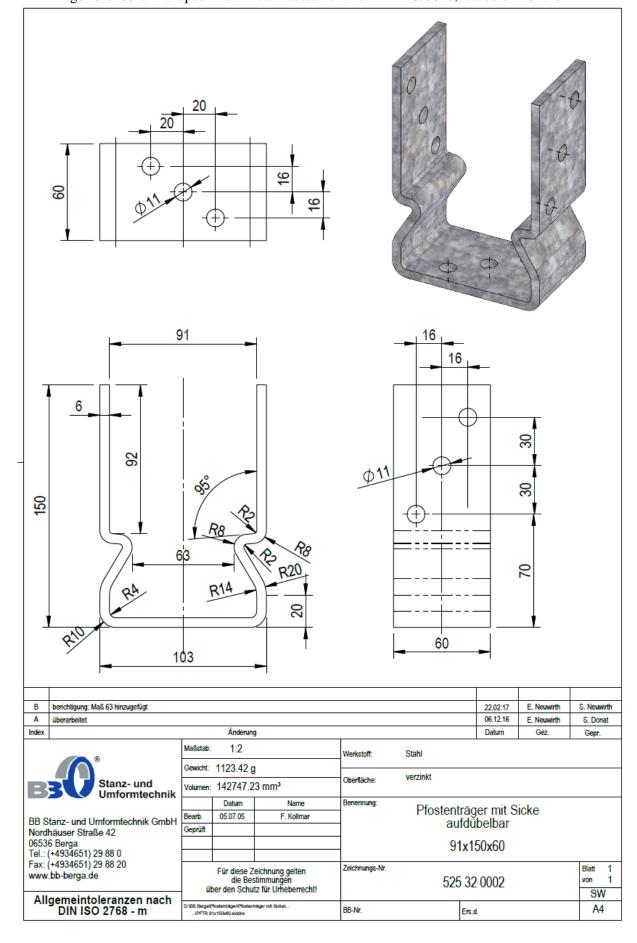


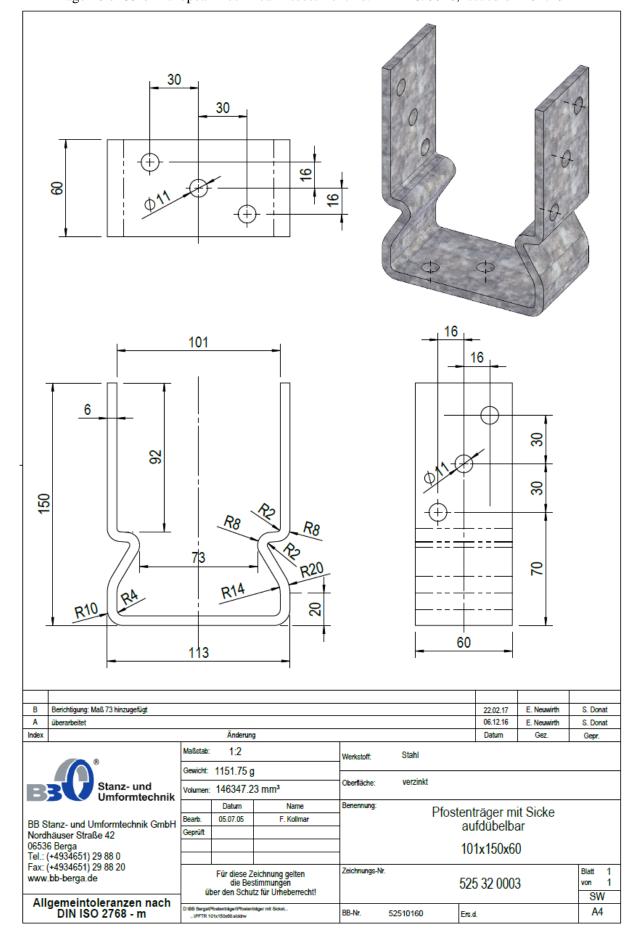




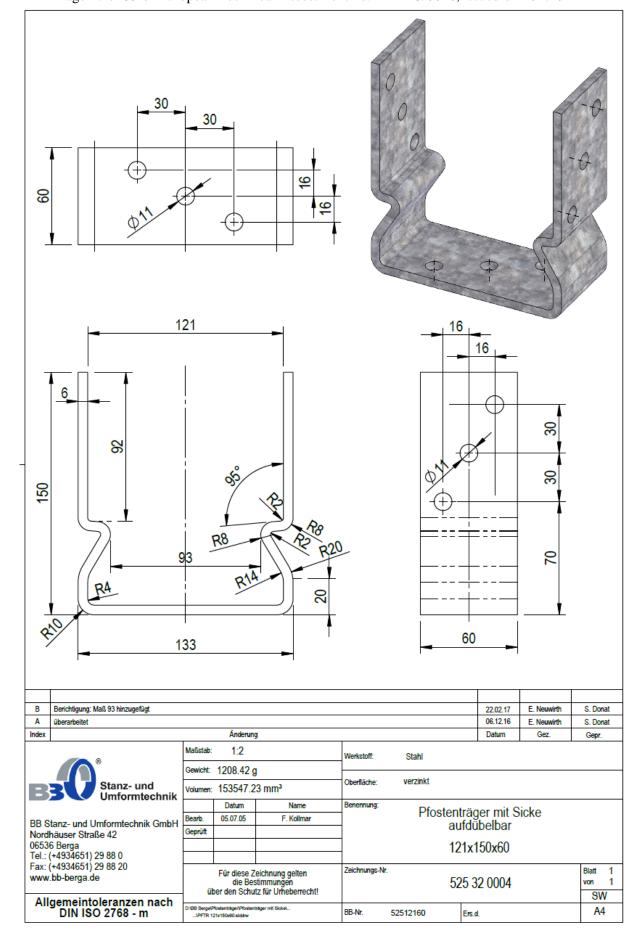


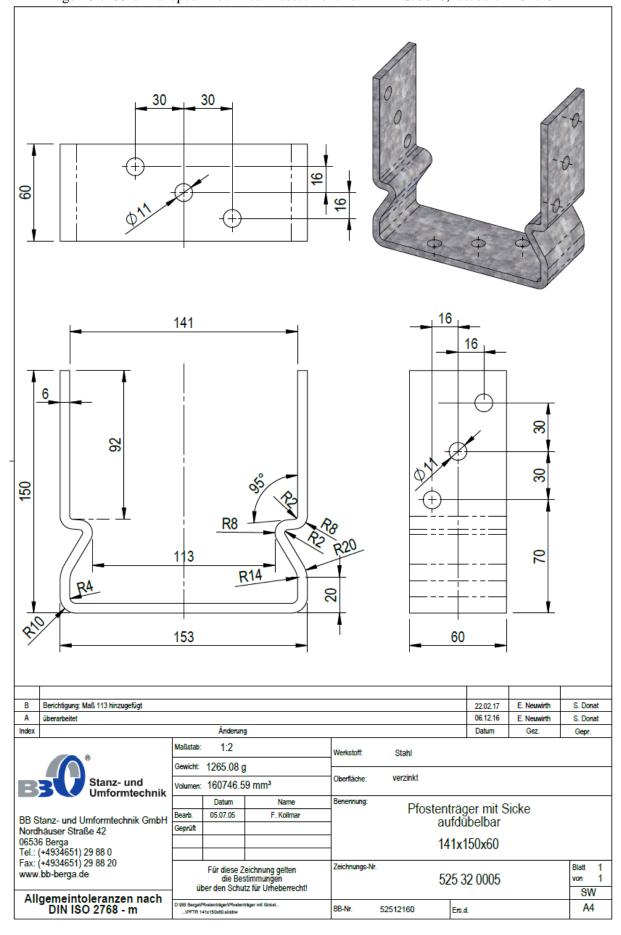




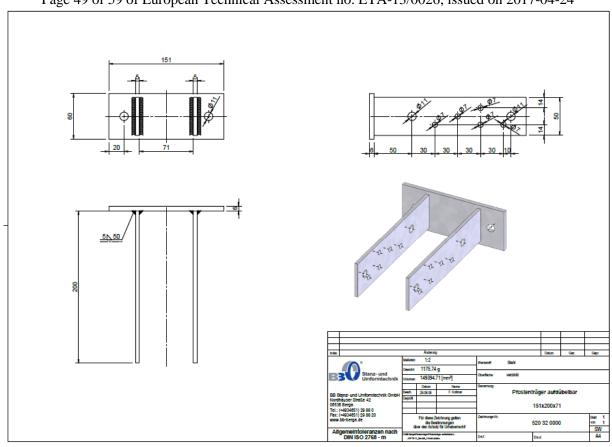


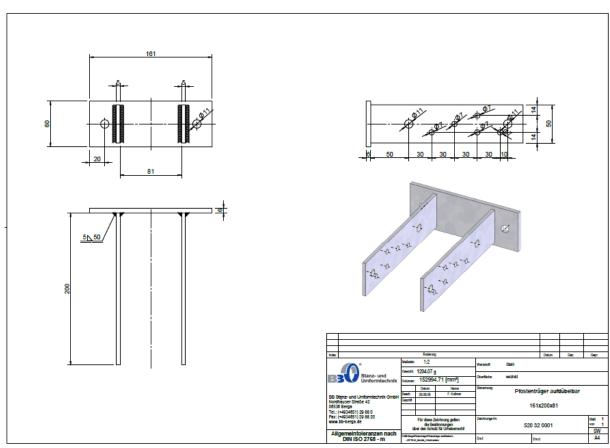
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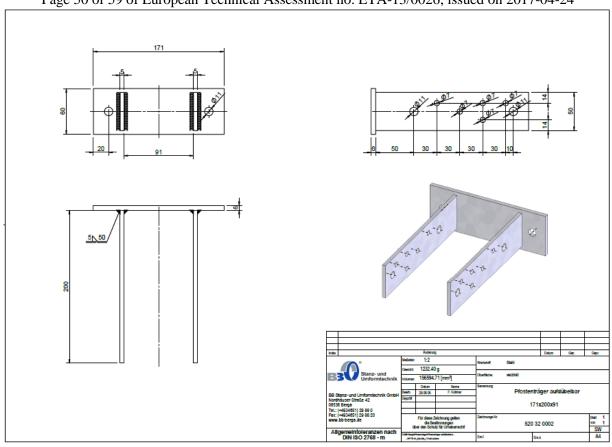


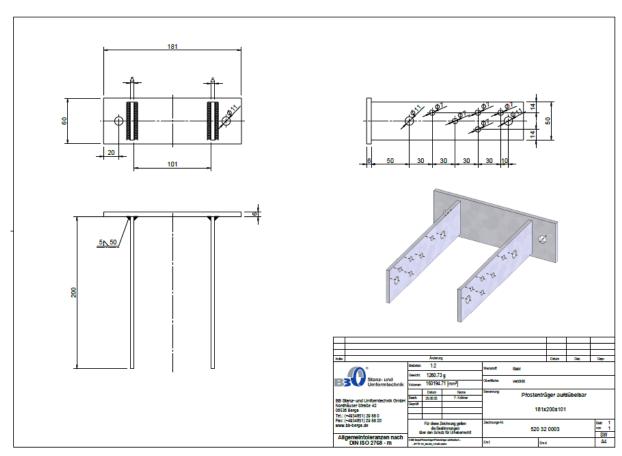
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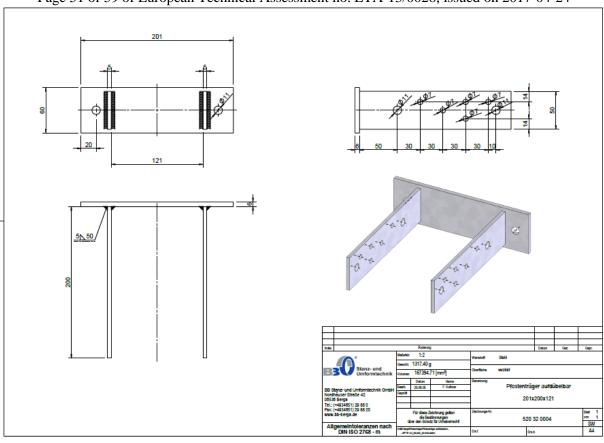


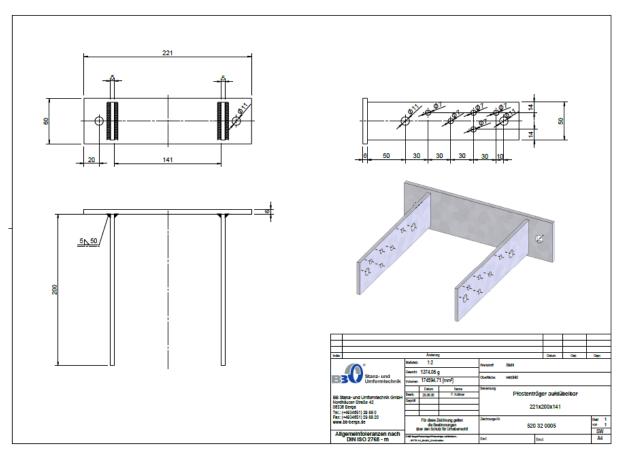
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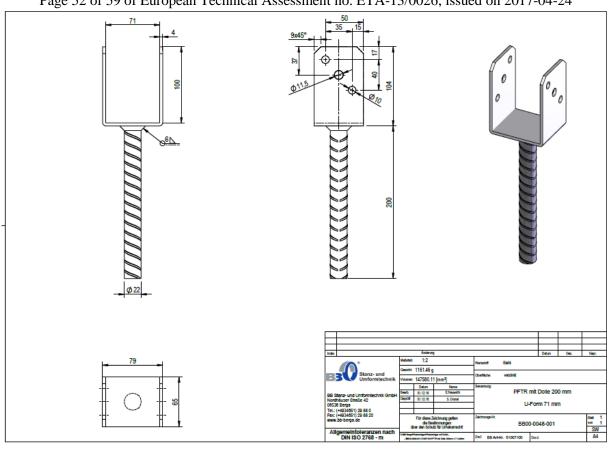


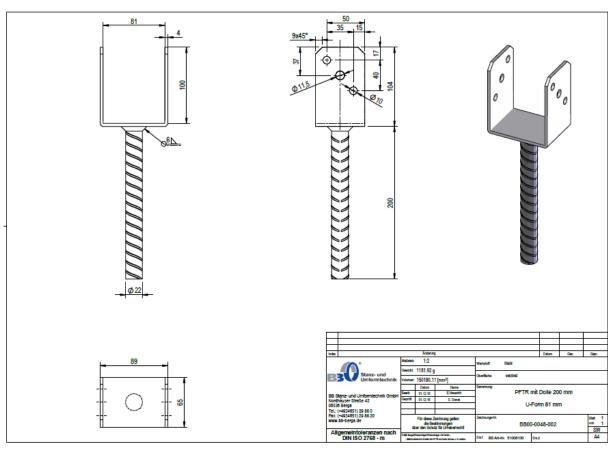
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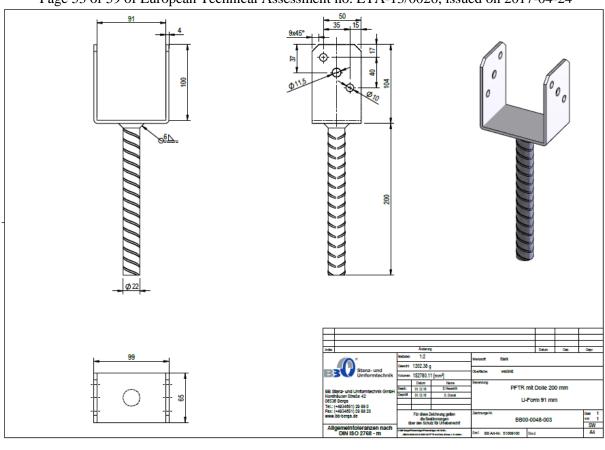


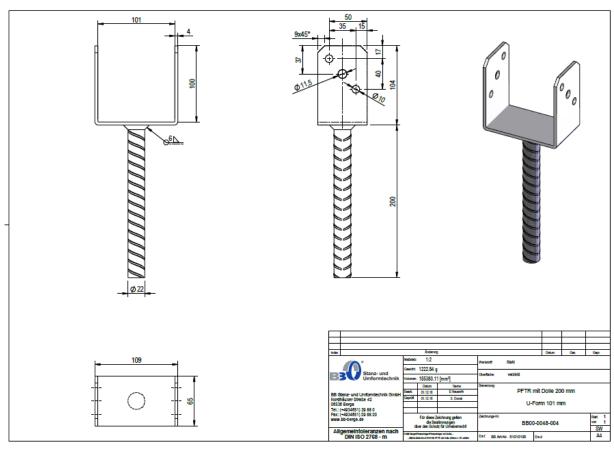
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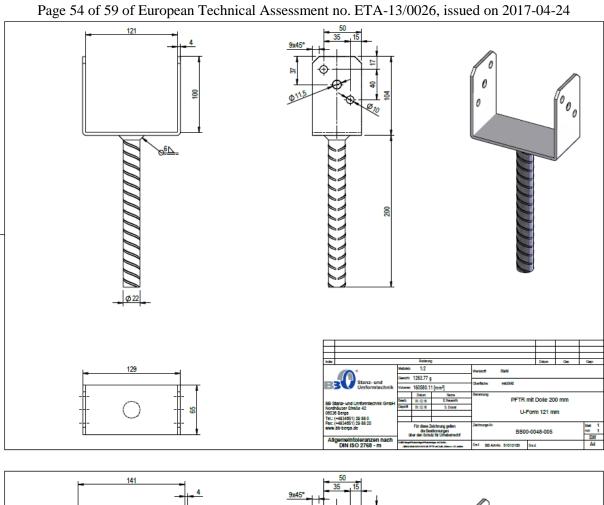


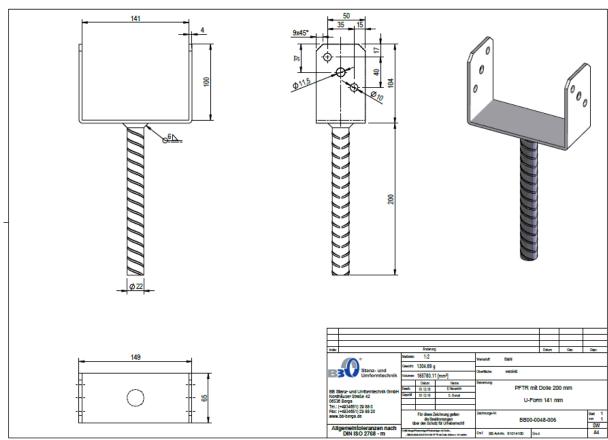


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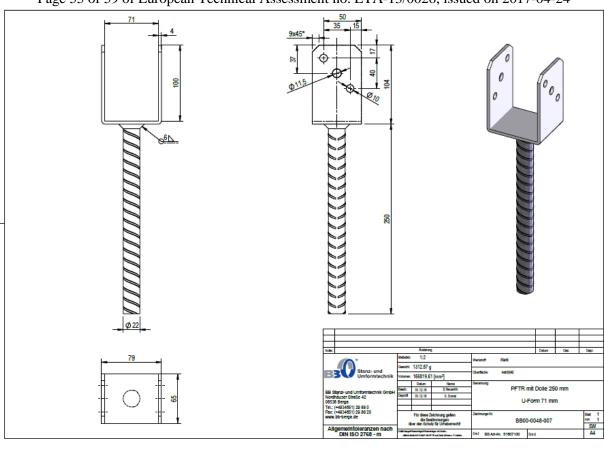


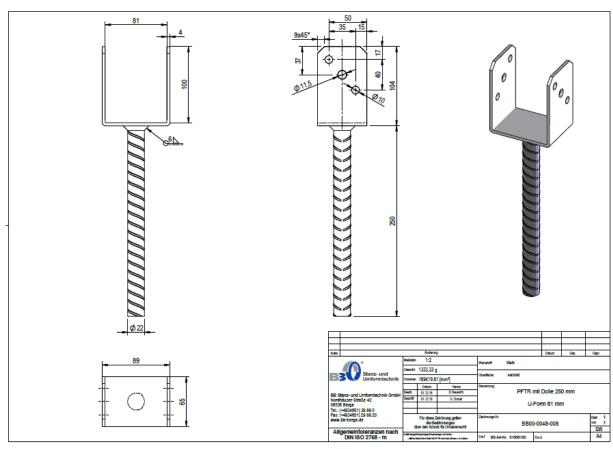




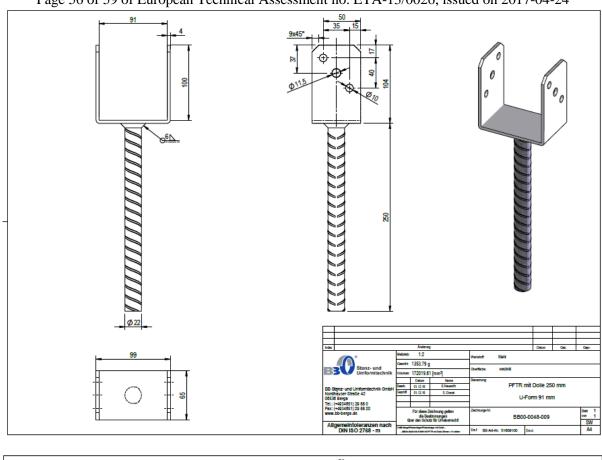


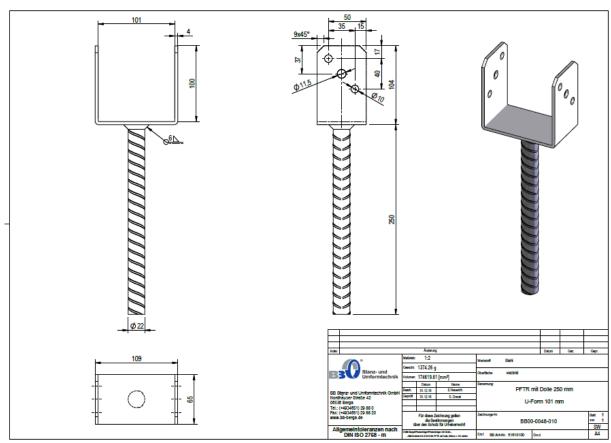
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